

ASPHALT

THE SMOOTH QUIET RIDE



2017 Local Roads Workshop

Project Scoping

March 2017



MICHIGAN RIDES ON US

Asphalt.

Project Scoping



Asphalt Pavements – America's Most Recycled Product

SMOOTH | DURABLE | SAFE | QUIET

Presentation Outline



- **Mix Selection**
- Rehab of Concrete Pavements
- Preventive Maintenance

Mix Selection – Mix of Fixes



Capital Preventive Maintenance - (Crack Filling, Surface treatments, thin overlays)

Rehabilitation - Conventional overlays, Rubblization, ASCRL, Etc.

Reconstruction - Full depth pavement removal and replacement

Mix of Fixes



Capital Preventive maintenance: Shorter Term Fixes

- 1 ½" HMA Overlay
- Milling and 1 ½" HMA Overlay
- Crack Treatment
- Overband Crack Filling
- Chip Seal
- Micro-Surfacing
- Ultra-Thin HMA Overlay

Mix of Fixes



Rehabilitation: Medium Term Fixes

- Structural HMA Overlay (Multiple Course)
- Mill and Structural HMA Overlay
- In-Place Recycling
 - Crush and Shape
 - Cold-in-Place
 - Hot-in-Place
 - Full Depth Reclamation
- Rubblization
- ASCRL

Mix of Fixes



Reconstruction: Long Term Fixes

- Full depth pavement removal and replacement
- New Construction

Selecting the Right Mix



- Selection of Mix for:
 - Optimum Performance
 - Economics
- Binder Selection Economics
- Lift Thickness vs. Performance
- Use of RAP

Binder Selection



Example:

- 1 1/2" resurfacing of existing road
 - 98% reliability binder grade is PG 64-28
- Consider using PG 64-22 ?
 - Reflective cracking

Lift Thickness based on Nominal Maximum Aggregate Size (NMAS)



NMAS – 1 size larger than the first sieve to retain more than 10%

NMAS



Table 2: Aggregate Properties

| | Mixture No. | | | | |
|------------|---|---------|---------|-------|--------|
| | 2C | 3C | 4C | 13A | 36A |
| | Percent Passing Indicated Sieve or Property Limit | | | | |
| 1 1/2 inch | 100 | | | | |
| 1 inch | 91-100 | 100 | | | |
| 3/4 inch | 90 max. | 91-100 | 100 | 100 | |
| 1/2 inch | 78 max. | 90 max. | 91-100 | 75-95 | 100 |
| 3/8 inch | 70 max. | 77 max. | 90 max. | 60-90 | 92-100 |
| No. 4 | 52 max. | 57 max. | 67 max. | 45-80 | 65-90 |
| No. 8 | 15-40 | 15-45 | 15-52 | 30-65 | 55-75 |
| No. 16 | 30 max. | 33 max. | 37 max. | 20-50 | |
| No. 30 | 22 max. | 25 max. | 27 max. | 15-40 | 25-45 |
| No. 50 | 17 max. | 19 max. | 20 max. | 10-25 | |
| No. 100 | 15 max. | 15 max. | 15 max. | 5-15 | |

Ex: 4C mix – NMAS is 1/2”

Local Agency Programs HMA Selection Guidelines



Local Agency Programs Hot Mix Asphalt (HMA) Selection Guidelines

Rev: 06/14/2016 - FHWA Approval: 08/29/2016

The following guidelines have been developed at the request of Local Agency Engineers for use on Local Agency projects. These guidelines have been reviewed and approved by the County Road Association of Michigan Engineering Committee. Previous experience and performance shall permit variations from these guidelines as per Section D. Alternative Mixes.

Local Agency Programs HMA Selection Guidelines



| Mixture Type | Marshall Mixture | | | | | Superpave Mixture | | | | |
|--------------|------------------|-----|-----|-----|-----|----------------------------|-----|-----|------|-----|
| | 36A | 13A | 2C | 3C | 4C | LVSP | 3E_ | 4E1 | 4E3+ | 5E_ |
| Min. #/syd | 110 | 165 | 350 | 220 | 165 | 165 Top or Leveling | 330 | 165 | 220 | 165 |
| Max. #/syd | 165 | 275 | 500 | 330 | 275 | 220 Top 250 Leveling | 410 | 330 | 275 | 220 |

Note: Application Rate of 110#/syd. Per 1 inch Thickness

Lift Thickness vs. Performance



- In-place Density is Critical
 - Initial In-place Air Voids <8%
- Lift Thickness Affects Compaction
 - Consolidation “Room”
 - Cooling Rate

Why Recycle RAP into HMA?



- **Best and Highest use**
- **Same or better performance as virgin mix**
- **Reduces demand for new materials**
- **Reduces carbon footprint**
- **Contains valuable materials**
- **Save \$**

Why Recycle RAP into HMA?



- RAP contains valuable materials :
 - ◆ Aggregate ~ 94% @ \$10/ton
 - ◆ Asphalt ~ 6% @ \$320/ton
 - ◆ Value = \$28.60 /ton (minus processing)

Economic Savings Example

- Aggregate: \$10.00/ton
- Asphalt: \$320.00/ton
- RAP: \$9.00
- Mix Design AC Content: 6.0%

| Material | 0% RAP | 17% RAP | 27% RAP |
|------------|---------|---------------|---------------|
| Aggregate | \$9.40 | \$7.70 | \$6.70 |
| Asphalt | \$19.20 | \$15.94 | \$14.02 |
| RAP | | \$1.53 | \$2.43 |
| Total | \$28.60 | \$25.17 | \$23.15 |
| \$ Savings | | \$3.43 | \$5.45 |
| % Savings | | 12.0% | 19.1% |

Recommended Practices for Use of RAP (and RAS)



Follow best practices for the processing and management of RAP

Contractor to sample and test RAP during processing

RAP usage specification

RAP mixes should meet same specs as virgin mixes

Adjust binder grade appropriately

Approved mix design including RAP

Recommended Practices for Use of RAP



Approved mix design including RAP

Know the properties of the RAP

- Gradation , binder content ,
theoretical maximum specific gravity

Mix design must be done incorporating RAP and taking into account the RAP characteristics

Mix Design Example



Michigan Department of Transportation form 1931 Report of Test File 300 _____
 SUPERPAVE™ HMA Design Mix Formula ACCEPTED

Distribution: Project Engineer (1) -- TMI (1) -- Mix Design (1) -- Contractor (1) -- Bit File (1)

| | | | | | | | | | | | | |
|--------------------------------|------------------------------|-----------------------------------|----------------------------------|------------------------|--------|-----|-----|-----|-----|-----|-----------------|-------------|
| Control Section B006 54038 | Job Number 54321A | Project Engineer R. Steel P.E. | Engineering Firm MDOT | Date 8/9/0X | | | | | | | | |
| Contractor General Pavement | | Plant Location BIG RAPIDS | Plant No. 701-01 | | | | | | | | | |
| Mix Type SE3 | Mix Design Number 96MD540 | Project Location | Specification 95SP501(F) | | | | | | | | | |
| % Air Voids 4.0 | VMA 15.9 | VFA 74.8 | P200/P ₂₅ 1.1 | AWI 288 | | | | | | | | |
| Gmm 2.457 | Gmb 2.359 | Gb 1.029 | Gsb 2.644 | Gse 2.682 | | | | | | | | |
| | | | | Film Thickness 7.21 | | | | | | | | |
| Pit Number | A 54-101 | B 54-101 | C 95-76 | D 95-76 | E | F | G | H | I | J | Plant Rap | % AC 5.7 |
| Aggregate Type | Sand FSU | Slag Sand | 810 FSU | Sand | 810 | | | | | | 15.0% | 2.48% |
| Blend % | 10.0% | 15.0% | 26.0% | 33.0% | 1.0% | | | | | | 15.0% | Combined |
| Sieve Size | GRADATION | | | | | | | | | | % Binder of RAP | 3.60 |
| 1 1/2" - (37.5mm) | | | | | | | | | | | | 0.0% |
| 1" - (25.0mm) | | | | | | | | | | | | 0.0% |
| 3/4" - (19mm) | | | | | | | | | | | | 0.0% |
| 1/2" - (12.5mm) | 100.0% | 100.0% | 100.0% | 100.0% | 100.0% | | | | | | 100.0% | 100.0% |
| 3/8" - (9.5mm) | 100.0% | 100.0% | 100.0% | 100.0% | 100.0% | | | | | | 87.5% | 98.1% |
| #4 - (4.75mm) | 91.3% | 91.7% | 65.0% | 99.9% | 100.0% | | | | | | 67.9% | 83.9% |
| #8 - (2.36mm) | 69.9% | 59.6% | 39.7% | 79.9% | 100.0% | | | | | | 50.2% | 61.1% |
| #16 - (1.18mm) | 52.4% | 38.7% | 29.2% | 66.2% | 100.0% | | | | | | 40.8% | 47.6% |
| #30 - (0.80mm) | 36.8% | 26.3% | 23.8% | 54.9% | 100.0% | | | | | | 33.6% | 38.0% |
| #50 - (0.30mm) | 11.8% | 18.0% | 17.2% | 24.9% | 100.0% | | | | | | 20.8% | 20.7% |
| #100 - (0.1mm) | 3.6% | 11.7% | 11.7% | 2.6% | 100.0% | | | | | | 10.6% | 8.6% |
| #200 - (0.075mm) | 2.5% | 7.7% | 9.0% | 0.4% | 85.0% | | | | | | 7.4% | 5.8% |
| 1 FACE CRUSH % | 30.0% | 100.0% | 100.0% | 30.0% | | | | | | | 75.0% | 88.6% |
| 2 FACE CRUSH % | | | | | | | | | | | | - |
| L.A. ABRASION & YEAR | 22-03 | 25-02 | 22-03 | | | | | | | | | |
| Angularity Index | 37.8 | 48.8 | 43 | 38 | | | | | | | | 40.90 |
| AWI FACTOR | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | 1.0 | | |
| AWI VALUE #16 | 225 | 401 | 300 | 240 | | | | | | | 240 | 288 |
| COMBINED Calc. Gsb | 2.601 | 2.720 | 2.610 | 2.648 | | | | | | | 2.675 | 2.648 |
| #4+ COARSE BULK S.G. | | | 2.621 | | | | | | | | | |
| #8 COARSE BULK S.G. | | 2.702 | 2.582 | | | | | | | | | |
| FINE BULK S.G. | 2.601 | 2.73 | 2.619 | 2.648 | | | | | | | 2.678 | |
| FLAT & ELONGATED % | | | | | | | | | | | | |
| SOFT PARTICLES % | 0.1 | | 0.5 | 0.5 | | | | | | | | 0.5 |
| REMARKS: | Asphalt Binder | Grade PG 64-28 | A.C. Supplier I.D. # ABS 1005 | % New AC Added 5.16 | | | | | | | | |

The bitumen content and aggregate characteristics are based on the submitted materials with the gradation and blend values indicated. Variations in materials or field conditions may require adjustments of this mix design from TMI for base or 1011 for field applications. The contractor should be notified of any necessary adjustments to this mix design and should not be applied or adjusted without written approval of the Bituminous Services Unit. A signed copy will be with the Bituminous Contract Unit. Tested for information.

Bituminous Engineer

| | | | |
|-----------------|---|---|----------|
| | H | I | J |
| | | | Plant |
| | | | Rap |
| | | | 15.0% |
| % Binder of RAP | | | 3.60 |
| | | | % AC |
| | | | 5.7 |
| | | | 2.48% |
| | | | Combined |
| | | | 0.0% |

| | | | |
|----------------|--|--|-------|
| | | | 2.678 |
| | | | 0.5 |
| % New AC Added | | | 5.16 |

Ability to Meet Volumetrics



- Treat RAP as mix component
- Challenges: VMA and dust/asphalt ratio
 - Good processing practices and quality control can be used to overcome this issue



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Recommended Practices for Use of RAP (and RAS)



Test the produced Mix:

(Binder, Gradation)

Contractor Quality Control Tests

Owner Quality Assurance / Acceptance tests

If you have performance concerns:

Consider testing/monitoring other properties

Mix volumetrics (Air Voids, VMA)

Fines to Effective Binder Ratio

Presentation Outline



- Mix Selection
- **Rehab of Concrete Pavements**
- Preventive Maintenance

Rehabilitation of Concrete Pavements



- Improve ride quality
- Correct surface defects
 - improve surface drainage
 - increase surface friction
- Delay/prevent structural deterioration
- Strengthen pavement structure (rehabilitation)

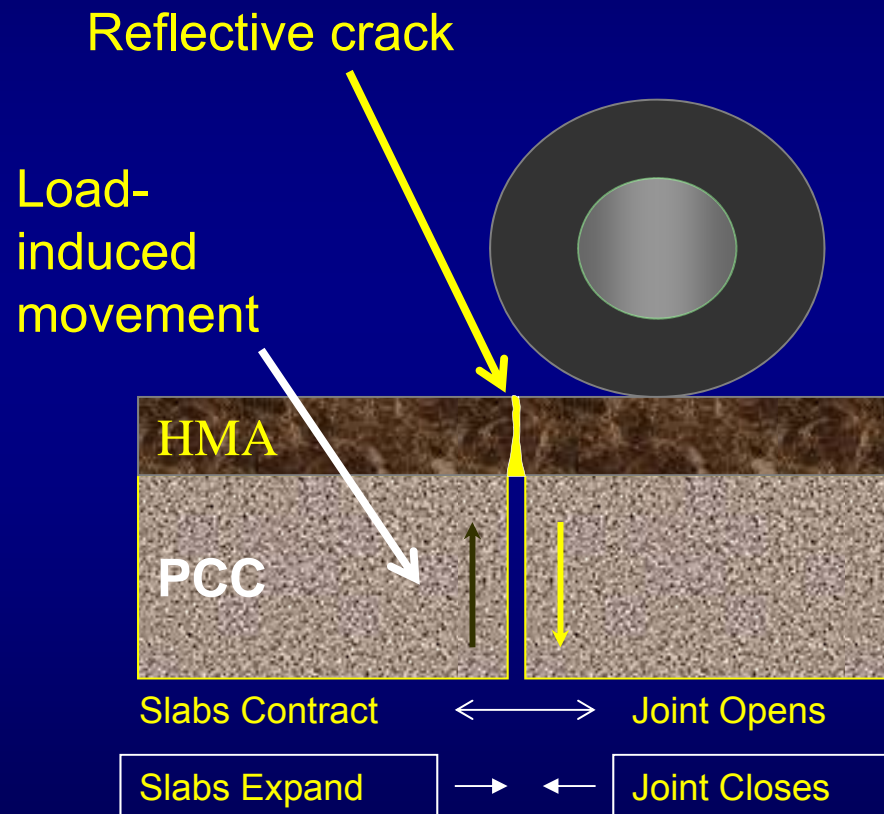
Rehabilitation Design Factors



- Condition of existing pavement
 - Drainage
 - Distress
 - Response to load
- Foundation strength/stiffness
 - Subbase
 - Subgrade
- Future traffic loading
- Additional corrections (safety, capacity, etc)

Reflective Cracking

- By far, the biggest issue in HMA overlays of PCC pavement
- Caused by movement at PCC joints and cracks



Design Issues



- Rate of propagation through overlay
- Number of reflected cracks
- Rate of deterioration of reflected cracks
- Amount of water that can infiltrate through the cracks

Reflection Crack Control Measures

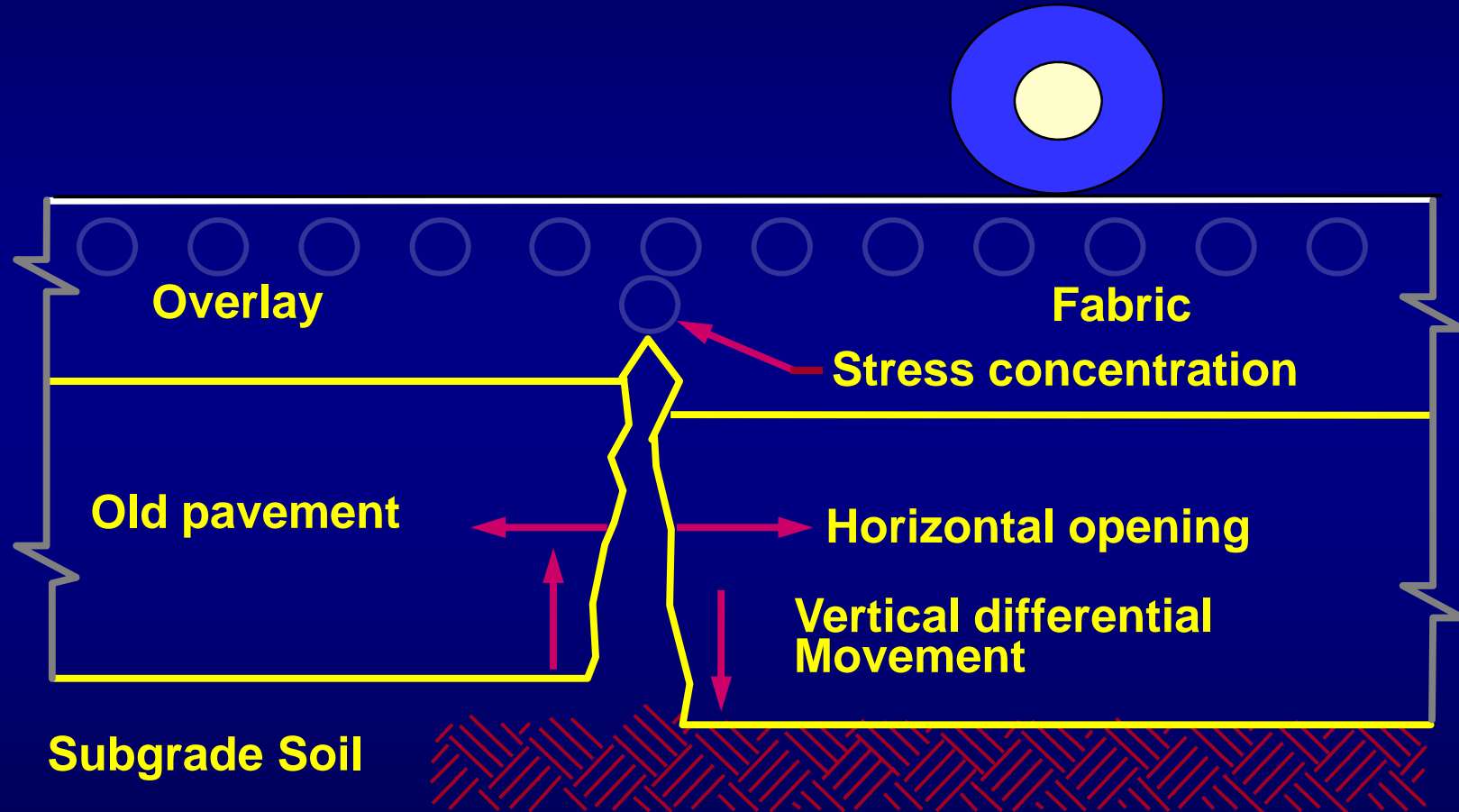


- Fabrics
- Stress-relieving interlayers
- Crack-arresting interlayers
- Pre-overlay treatments
- Slab repair or replacement
- Sawing and sealing joints
- Increased overlay thickness

Crack Control Effectiveness

- Delay the occurrence of cracking
- Reduce the number of cracks
- Control the crack severity
- Provide other benefits
 - Reduce overlay thickness
 - Enhance waterproofing capabilities

Fabrics



Fabric Application



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Pre-overlay Repairs



- Slab stabilization
- Fractured slabs
- Slab repair / replacement
- Load transfer restoration

Fractured Slab Techniques



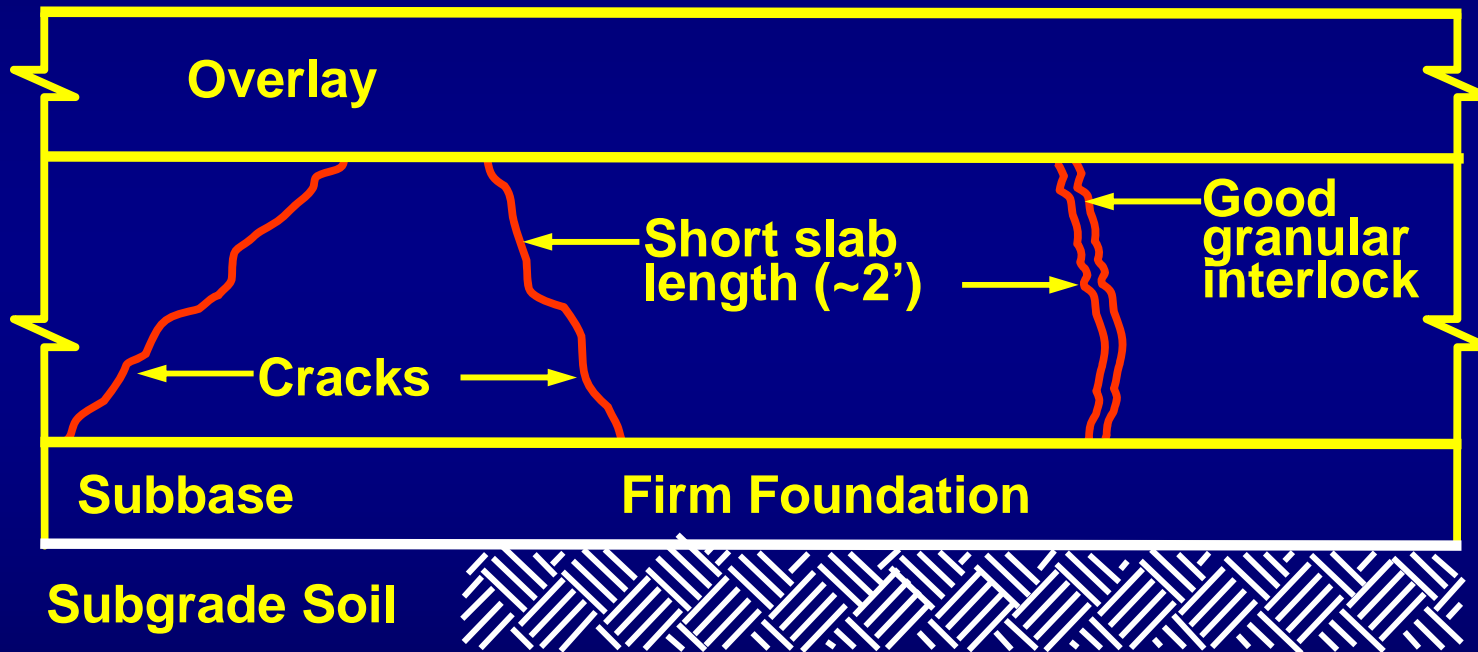
- Crack and seat (JPCP)
- Break and seat (JRCP)
- Rubblize (JPCP, JRCP, CRCP)

Cracking and Seating



- Shortens effective slab length
- Standard practice in many States
- Not “generally” recommended for use on poor subgrades
- Design methods (overlay thickness)

Cracking and Seating



Cracking and Seating



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Cracking and Seating



HMA Maint. & Rehab.

37

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Rubblization



- **Fracturing:**
 - Eliminates slab action
 - Destroys bond between concrete and steel
- **Rubblized base responds as a tightly keyed, interlocked high-density, unbound layer**
 - Layer cannot crack; already fractured

Why Rubblize?

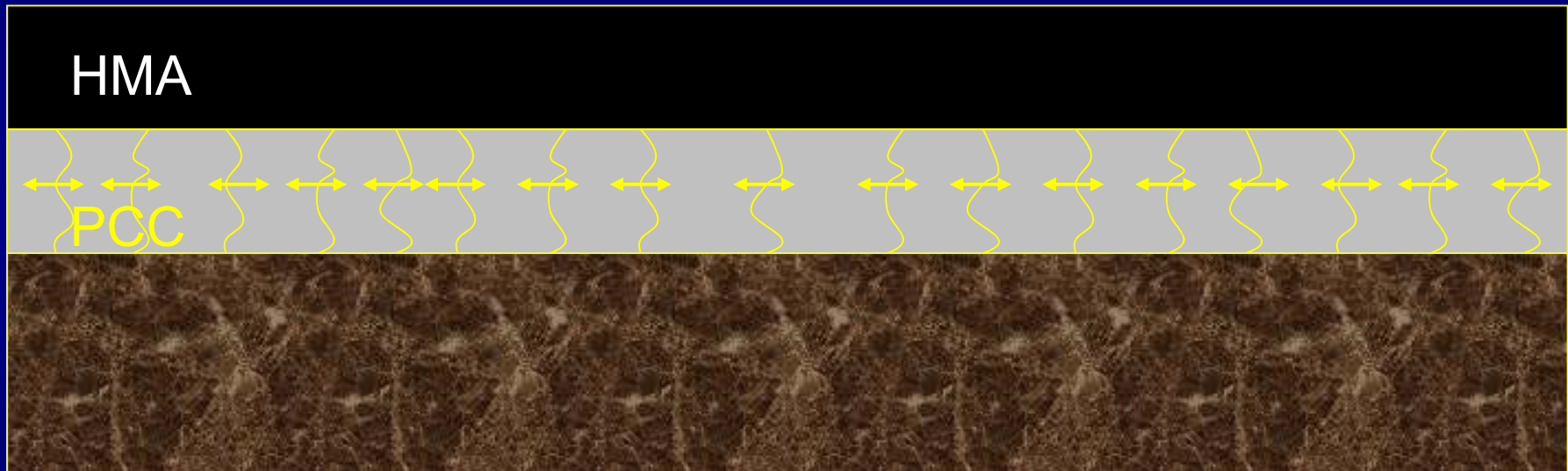


- Fracturing PCC to segments less than 9” precludes reflection of:
 - Joints
 - Cracks
 - Faults
- Production Rates up to 1 lane-mile/day

Rubblization



Smaller Pieces = Smaller Movement = No Cracking



Rubblization - Equipment

Resonant Pavement Breaker



Multi-Head Breaker (MHB)

When to Rubblize

- Patching \geq 10%.
- Severe D-cracking.
- Severe ASR or ACR cracking.
- Dowel bar locking
- Severe joint deterioration
- Persistent faulting.

Precaution



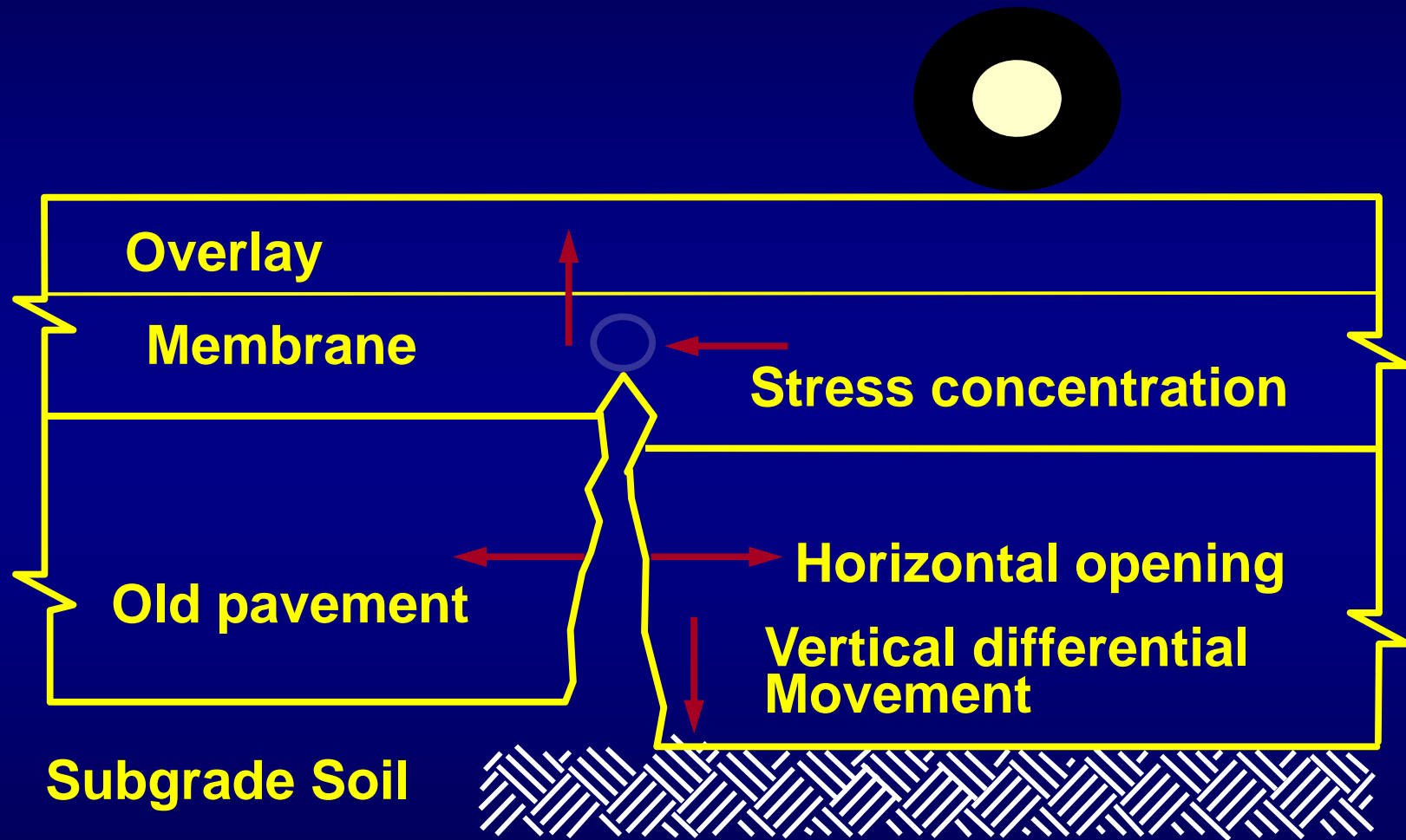
- Weak soils may make construction difficult.
- **Option 1**
 - Adjust breaking pattern (12 - 18") in soft areas.
 - Use normal seating rolling.
 - Resume smaller pattern after weak area.

Precaution

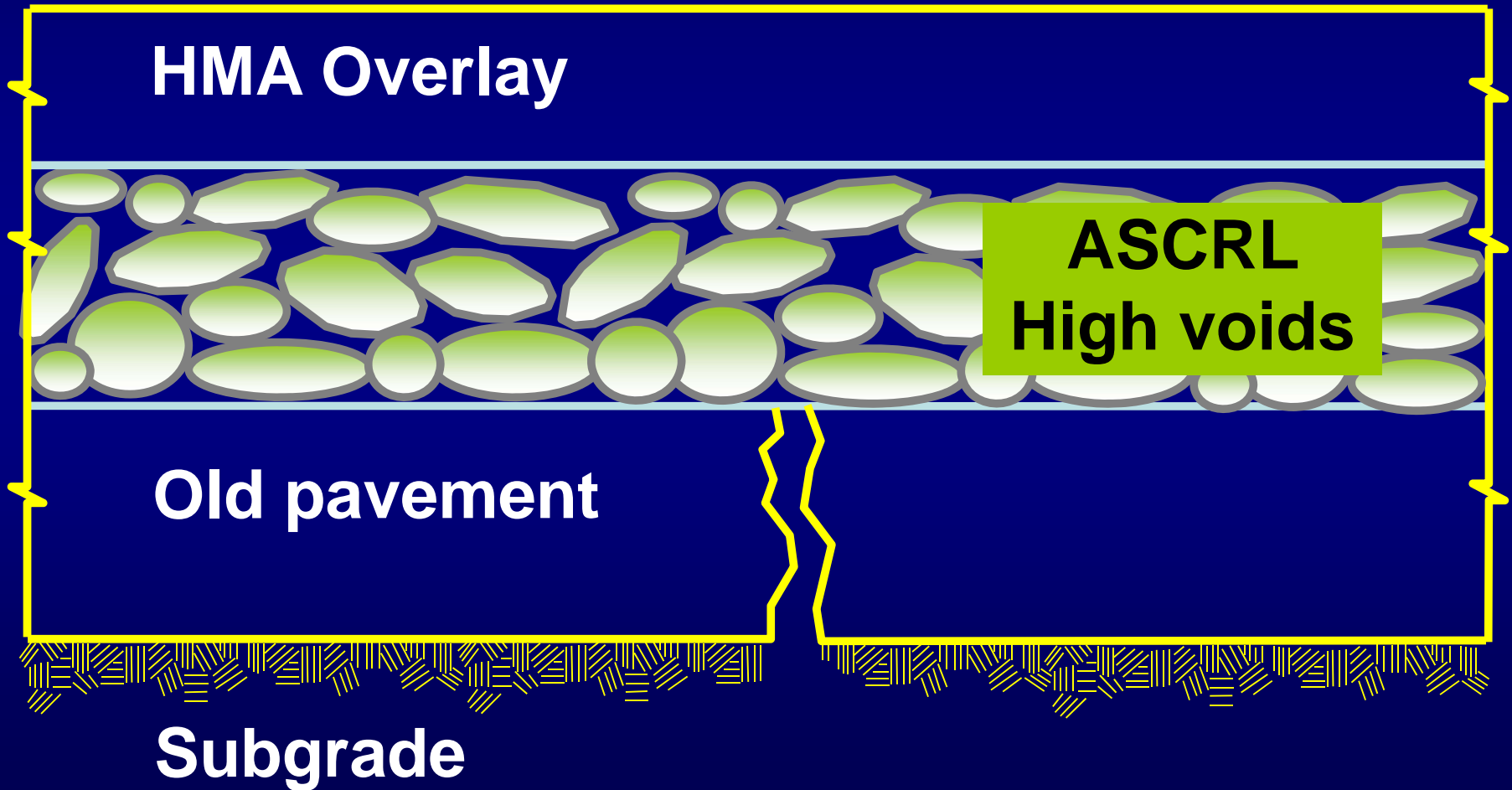


- Option 2
 - Cease rubblization
 - Define weak area
 - Remove/replace problem material
 - Resume normal operations when past weak area
- Perform a good soils evaluation prior to construction

Stress-Absorbing Interlayers



Asphalt Stabilized Crack-Relief Layer (ASCRL)



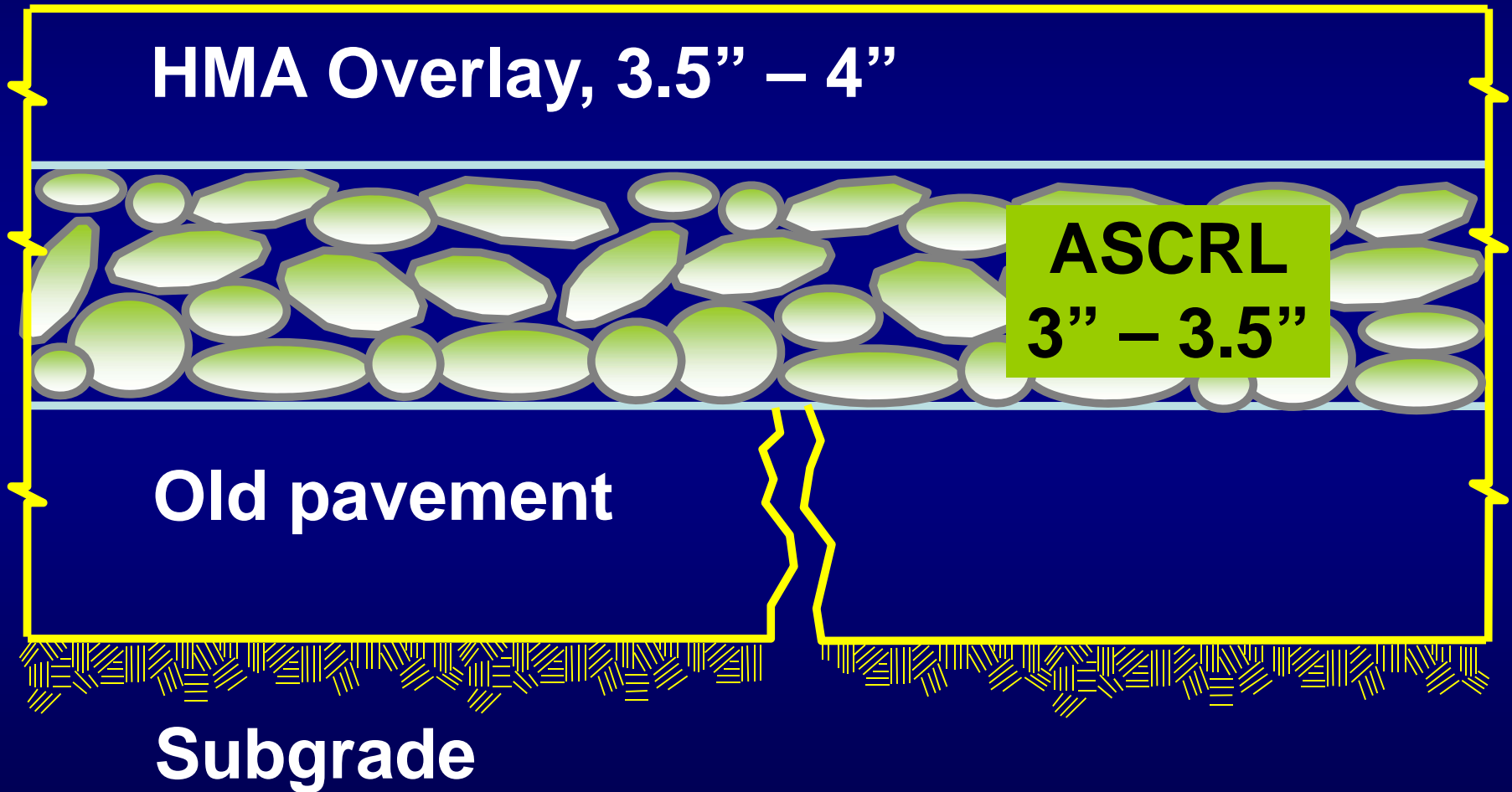
Asphalt Stabilized Crack-Relief Layer (ASCRL)



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Asphalt Stabilized Crack-Relief Layer (ASCRL)



Asphalt Stabilized Crack-Relief Layer (ASCRL)



MICHIGAN
DEPARTMENT OF TRANSPORTATION

SPECIAL PROVISION
FOR
ASPHALT STABILIZED CRACK RELIEF LAYER

C&T:GMM

1 of 4

C&T:APPR:JB:CJB:04-03-04

a. **Description.** Furnish, place and compact an asphalt stabilized crack relief layer (ASCRL) on a prepared pavement base according to the details shown on the plans or as directed by the Engineer. The HMA mixture will be provided according to the requirements of the 2003 Standard Specifications for Construction, except where modified herein.

b. **Materials.** The aggregate materials used to prepare the ASCRL shall meet the following requirements.

The coarse aggregate shall originate geologically only from natural sources. Crushed concrete or reclaimed asphalt pavement cannot be used in the ASCRL mixture.

Table 1 Aggregate Specifications

| Gradation Requirements | | | | | | |
|--|-------|--------|-------|-------|--------|----------------------|
| Sieve Size (inch) | 1 1/2 | 1 | 1/2 | No. 4 | No. 30 | No. 200 (LBW) (a) |
| Percent Passing | 100 | 90-100 | 30-60 | 10-25 | 5-15 | 3-5 |
| Physical Requirements | | | | | | |
| Crushed Material, Min. (MTM 117) % (b) | | | | 95 | | |
| Loss, max., Los Angeles Abrasion (AASHTO T96) % | | | | 35 | | |
| Soft Particle (max) % (c) | | | | 5.0 | | |
| a. Loss by Washing shall be by MTM 108. Mineral filler may used to meet the required percentage. | | | | | | |
| b. The percentage of crushed material will be determined on that portion of the sample retained on all sieves down to and including the No. 4 sieve. | | | | | | |
| c. The sum of aggregate particles retain on the No. 4 sieve identified as shale, siltstone, clay ironstone and particles which are structurally weak or are found to non-durable in service. | | | | | | |

c. **Mix Design.** The Contractor shall provide a mix design in accordance with the criteria herein. The following are the requirements for the testing, documentation, and material samples for a mix design verification. Submittal of the Mix Design and samples shall be made to MDOT.

Asphalt Stabilized Crack-Relief Layer (ASCRL)



Table 1 Aggregate Specifications

Gradation Requirements

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Asphalt Stabilized Crack-Relief Layer (ASCRL)



Mix Design

- Asphalt Binder – PG 64-28 with 0.5% liquid antistripping additive
- Asphalt content – 3 to 4 %
- Surface Coating – 100 % without excessive draindown (max 0.30 %)
- Minimum Asphalt film thickness - 9.0 microns
- Moisture sensitivity (AASHTO T283)

Asphalt Stabilized Crack-Relief Layer (ASCRL)



Construction

- Placed in a single layer
- Compaction – steel – wheeled tandem roller (1.0 ton per foot of drum length)
 - Static mode only
 - Minimum of three passes (down and back)
 - Compaction test strip may be required (minimize breakage of Agg.)

Asphalt Stabilized Crack-Relief Layer (ASCRL)



MDOT Projects to Date

| Project | # of Jobs | Length (miles) |
|---------|-----------|----------------|
| ASCRL | 44 | 150 |

Started in 1999

- All are performing very well

Asphalt Stabilized Crack-Relief Layer (ASCRL)



M-21, Before Construction

Asphalt Stabilized Crack-Relief Layer (ASCRL)



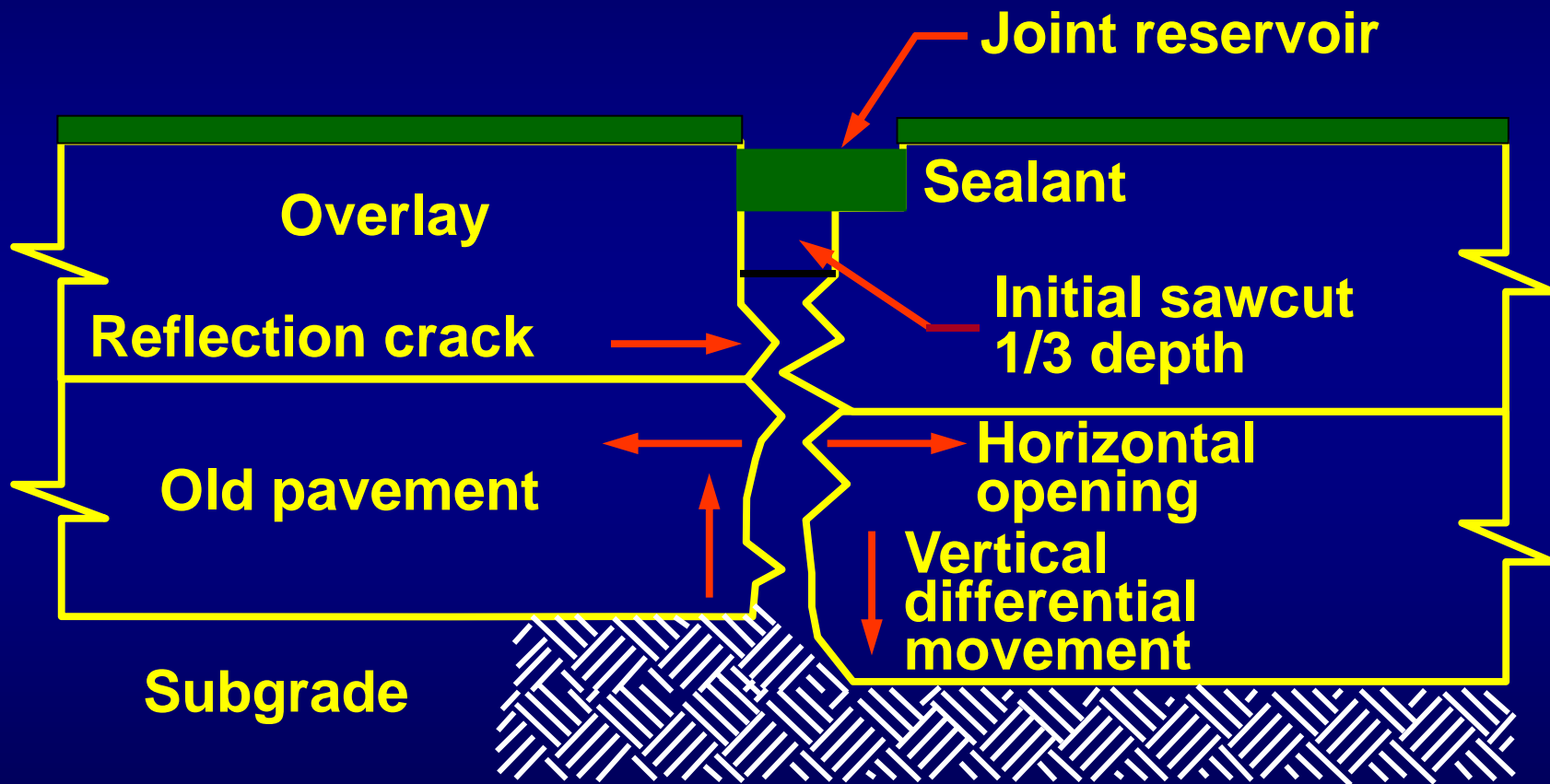
M-21, 3 years old

Sawing and Sealing Joints



- Concede appearance of reflection cracking
- Objective: control rate of deterioration
- Reduces spalling of reflection cracks
- Candidates should have well-defined joints
- Sawcut must be directly above underlying joint

Sawing and Sealing Joints



Sawing and Sealing Joints



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Increased Overlay Thickness



- Delays occurrence of reflection cracking
- Cracks propagate about 1 in. per year
- Reduces temperature fluctuations in underlying pavement

Presentation Outline



- Mix Selection
- Rehab of Concrete Pavements
- **Preventive Maintenance**

Preventive Maintenance



**WHAT EXACTLY IS YOUR
PREVENTIVE MAINTENANCE
PROGRAM PREVENTING?**

Preventive Maintenance



Preventive maintenance is a planned strategy of cost effective treatments to an existing roadway system and its appurtenances that preserves the system, retards future deterioration, and maintains or improves the functional condition of the system without substantially increasing structural capacity.

Preventive Maintenance



Standard Capital Preventive Maintenance Treatment Pavement Seal

- HMA Crack Treatment
- Overband Crack Fill- Pretreatment
- Chip Seals
- Micro-surfacing
- Ultra-Thin HMA Overlay-Low & Medium Volume (<1" thick)
- Shoulder Fog Seal
- Paver Placed Surface Seal

Preventive Maintenance



Crack Treatment



Preventive Maintenance



Chip Seal



Preventive Maintenance



Micro-surfacing



Preventive Maintenance



HMA Ultra-Thin High Value Pavement Enhancement



Extends pavement life
Protects pavement structure
Restores pavement smoothness

HMA Ultra-Thin



$\frac{3}{4}$ " to 1" Thickness



HMA Ultra-Thin

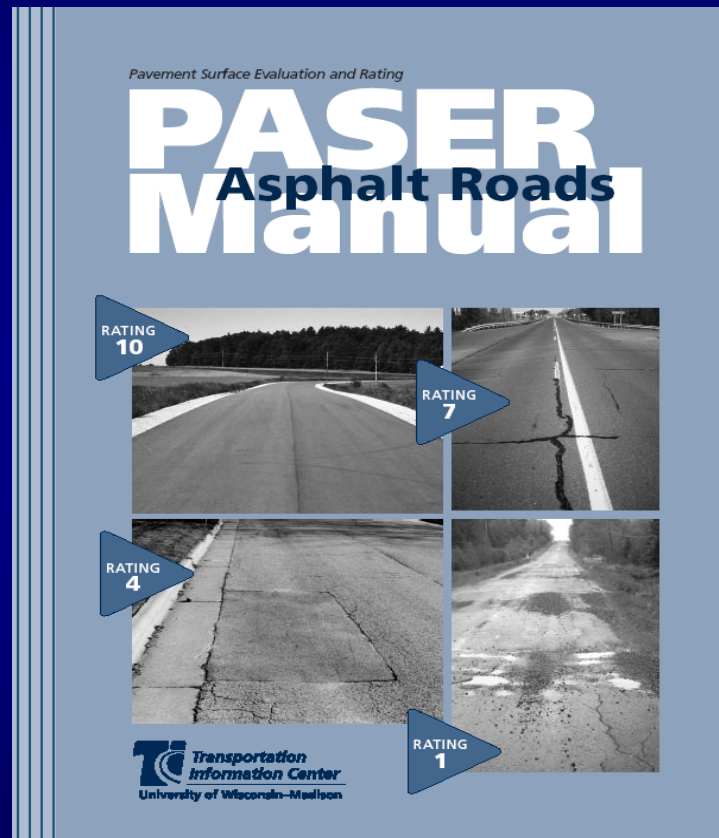


BENEFITS

- 5-9 year life extension
- Seals pavement to delay further deterioration
- Improve ride quality
- Minimize or eliminate structure adjustments
- Reduce noise
- Improve drainage
- Ease of construction, standard paving operation
- Minimal construction time



HMA Ultra Thin



4 – 6 Paser rating

HMA Ultra-Thin Performance



MDOT Projects - Statewide

| UT Type | # of Jobs | Length (miles) | Avg. Age (end service) | Avg. Age (in service) | Avg. Age (overall) |
|-----------------|-----------|----------------|------------------------|-----------------------|--------------------|
| Ultra-thin low | 52 | 483 | 7.6 (20) | 9.8 | 8.6 |
| Ultra-thin med | 41 | 339 | 5.5 (4) | 8.6 | 6.6 |
| Ultra-thin high | 16 | 89 | 5.3 (3) | 7.3 | 6.4 |

HMA Ultra-Thin Statewide



Preventive Maintenance Treatment Cost Comparison

| Treatment | \$/syd | Cost/mile (24' wide) | MDOT Life extension range (years)* | MDOT Life extension range average (years)* | Cost/mile* per year |
|------------------|--------|-------------------------|--|--|------------------------|
| Double chip seal | \$3.18 | \$44,773 | 3-5 | 4 | \$11,193 |
| Micro-surface | \$2.61 | \$36,747 | 3-5 | 4 | \$9,187 |
| Ultra-thin low | \$2.51 | \$35,339 | 5-9* | 9* | \$3,927 |
| Ultra-thin med | \$2.87 | \$40,408 | 5-9* | 8* | \$5,051 |
| Ultra-thin high | \$3.29 | \$46,321 | 5-9* | 7* | \$6,617 |

*Average Life Extension estimated by APAM

Unit Prices based on MDOT Information (thru Jan. 2016)

HMA Ultra-Thin



HMA UT Chip Seal Microsurfacing

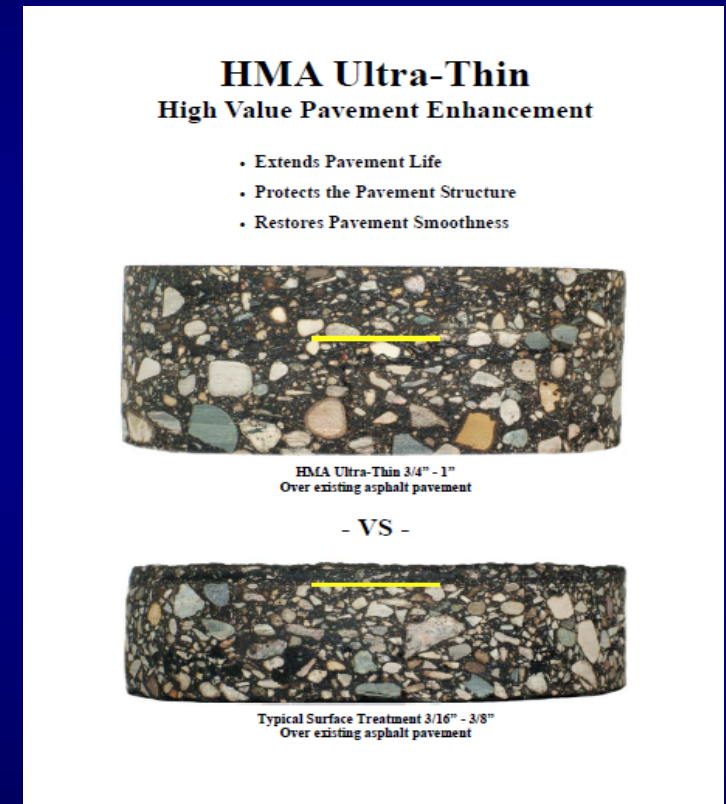
| | HMA UT | Chip Seal | Microsurfacing |
|----------------------------------|--------|-----------|----------------|
| Increase skid resistance | ✓ | ✓ | ✓ |
| Minimizes curb loss | ✓ | ✓ | ✓ |
| Corrects surface distress | ✓ | ✓ | ✓ |
| Can be applied in one pass | ✓ | ✓ | |
| Increases structural strength | ✓ | | |
| Improves ride quality | ✓ | | |
| Improves pavement draining | ✓ | | |
| Corrects minor rutting | ✓ | | ✓ |
| Eliminates dust, loose aggregate | ✓ | | ✓ |

HMA Ultra-Thin



Advantages:

- Adds structural value
- Very smooth riding surface
- Improved ride quality
- No excess stone buildup
- No broken windshields from loose aggregate



Project Scoping



Questions?